



# Introducing the New England Stormwater Retrofit Manual

NEIWPCC 2022 - The Clean Water Act: Past and Future

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#### **Manual Team and Manual Funding**

#### Presenting Today:



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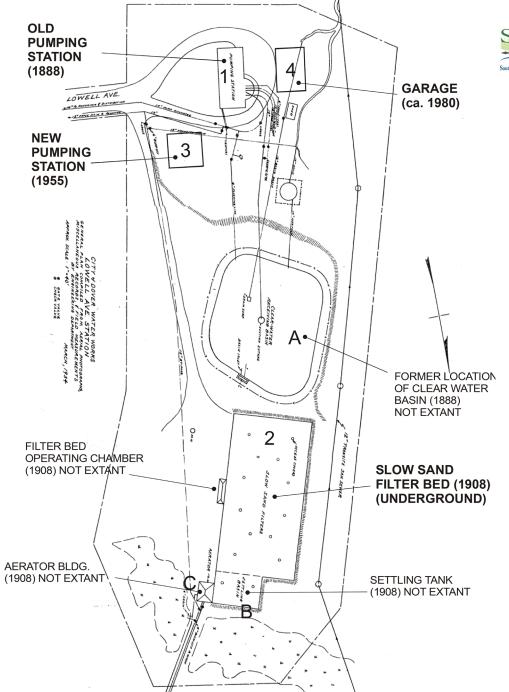








Historic Municipal Water Works,





Historic Municipal Water Works

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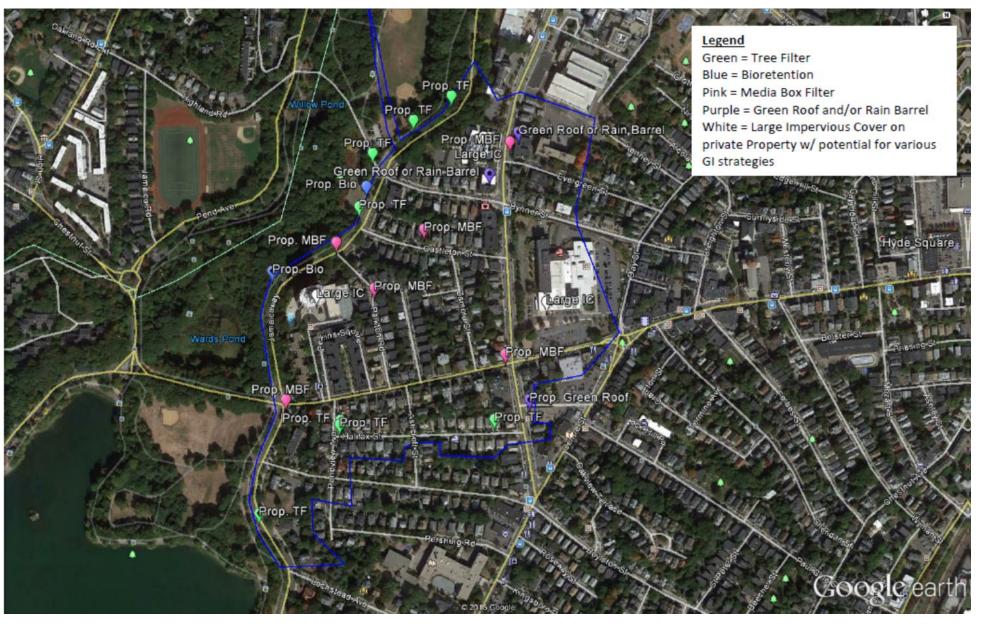






#### Where did the idea of a manual come from?





#### Watershed Approach

Location	Best Management Practice	Rationale	SNEÌ
Along west side of Jamaica Way	Tree Filters (5)	Existing storm drain line and several catch basins. There are also some recently planted trees that could be transplanted into tree filter units. These would provide some treatment through filtration, infiltration, rhizosphere, and flow attenuation. Media amendments would be added to remove phosphorus.	Southeast New England Progr
Halifax St and Pondview Ave	Tree Filters (2)	There is an existing storm drain line and catch basins on each side of the Halifax St. The existing street scape includes trees.	
Halifax St near S. Huntington St	Tree Filter	There is an existing storm drain line and a catch basin that could be converted to a tree filter.	
<ul> <li>Parkwood Terrace and Jamaica Way</li> <li>Perkins St and Jamaica Way</li> </ul>	Media Box Filters	Existing storm drain line and catch basin. Large vegetated strip between road and sidewalk that is likely in City right-of-way. Large trees are in this proximity so an MBF would be a better fit.	
<ul> <li>North end of Parkton Rd</li> <li>Perkins St and S. Huntington</li> <li>Castleton St</li> <li>Evergreen St and S. Huntington</li> </ul>	Media Box Filters	Existing storm drain line and catch basins on each side of Perkins St. No vegetated area but an MBF might fit under the road depending on other utilities.	
Just south of Highland Rd and Jamaica Way intersection	Bioretention	Existing storm drain line and room for a bioretention cell that could treat surface runoff and then be piped into existing line.	
West of Jamaica Way near Wards Pond	Bioretention	Plenty of area for a bioretention cell but some trees may need to be removed. Effluent could discharge cool treated water to Wards Pond.	
<ul> <li>El Mundo Newspaper</li> <li>Intersection of Bynner St and South Huntington</li> <li>Intersection of Evergreen St and South Huntington</li> </ul>	Green Roof and Rain Barrel installations	These are flat roof buildings with concrete block or brick exteriors, which may be strong enough for green roof systems. There are several other buildings in the drainage area that could be identified if this is a viable strategy.	
<ul> <li>MSPCA, Animal Care &amp; Adoption Center</li> <li>Perkins Square – Sagamore Advisors</li> <li>Mt. Pleasant Home</li> </ul>	Tree Filters, Bioretention(s), media box filters	Three of the largest, privately owned impervious cover areas in the watershed. There are media strips, large parking areas, and large roof tops that could all be managed using various GI strategies.	





#### And then reality



"Hi, Tim and I were just chatting about siting systems. Is there any reason why we could not put a system where the orange oval is in the pic below?"

"I'm going to say almost definitively no. It's private property and we have no way to get those property owners to work with us. Additionally, my understanding is that we want a visible area for public education (a park in this tributary area). "

### **Other Considerations**



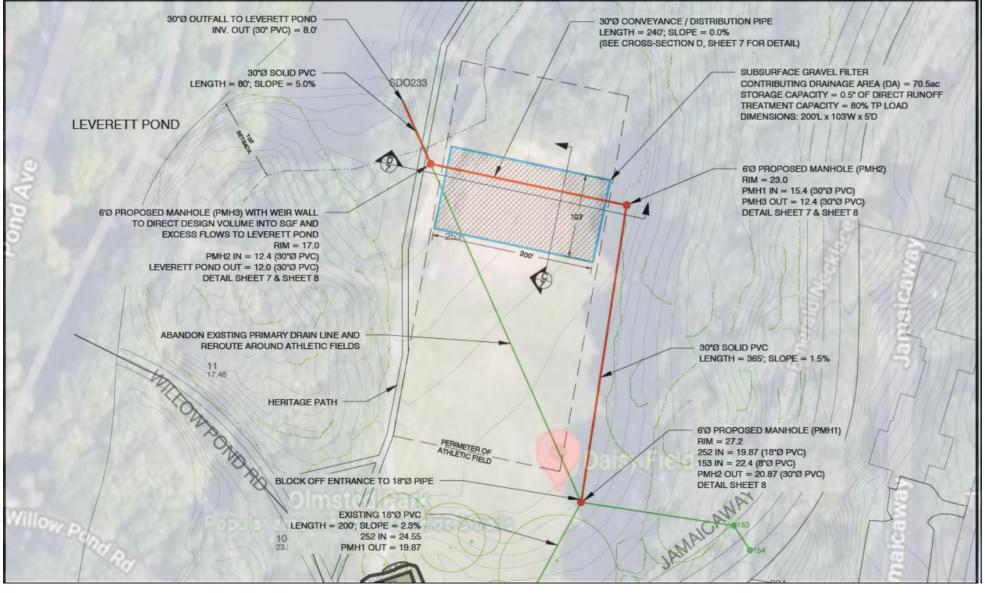
In the "TOPOGRAPHY" layer there are several sub-layers that seem to be paired for identical points on the map. They consistently differ by 6.5'. Which surface elevation layer is correct or relative to the Pipe Invert Elevation layers? For example:

Sub-Layer Name	Elevation	Elevation	Elevatio n	General Location of Point
Topo DETBCB Elevation	29.0	34.8	28.3	East of Jamaica Way / Willow Pond Rd intersection.
Topo DET Elevation	22.5	28.3	21.8	Same as above
Difference	6.5	6.5	6.5	
Topo INDBCB Elevation	26.5			On contour line southeast of Jamaica Way / Willow Pond Rd. int.
Topo INDD Elevation	20			Same as above
Difference	6.5			
<b>Topo BCB Elevation</b>	33.0	31.8	34.2	In vicinity of Jamaica Way / Willow Pond Rd. intersection
TOP GEN Elevation	26.5	25.3	27.7	Same as above
Difference	6.5	6.5	6.5	









Infiltration Trench (IR = 1.02 in/hr) BMP Performance Table: Long-Term Phosphorus Load Reduction									
BMP Capacity: Depth of Runoff Treated from Impervious Area (inches)0.10.20.40.60.81.01.5							2		
Runoff Volume Reduction	26.3%	44.6%	68.2%	81.0%	88.0%	92.1%	96.5%	98.	
Cumulative Phosphorus Load Reduction	27%	47%	73%	86%	92%	96%	99%	10	



Figure 3- 4: BMP Performance Curve: Infiltration Trench (infiltration rate = 1.02 in/hr)	Stormwater Management Design - 70.5 acre Ultra-Urban Drainage Area							
	Sizing Comparison of Capital Costs and Relative Phosphorus Load Removal Efficiency							
BMP Performance Curve: Infiltration Trench (infiltration rate = 1.02 in/hr)	Best Management Practice Size	Depth of Runoff Treated from Impervious Area (in)	*Storage Volume Cost (\$/ft³)	**Total Phosphorus Removal Efficiency (%)				
80% 80% 70% 5	Subsurface Gravel Filter - Minimum Size	0.35	\$1,016,912	62%				
60% 60% 60% 50% 50% e	Subsurface Gravel Filter - Moderate Size	0.5	\$1,452,732	80%				
40% 40%	Subsurface Gravel Filter - Full Size	1.0	\$2,905,463	96%				
a     30%     30%     30%       20%     20%     20%	*Storage Volume Cost est	imates provided by	EPA-Region 1 for Opti-To	ool methodology, 2015-Draft				
10% 10%	**Total Phosphorus %RE based on Appendix F Massachusetts MS4 Permit							
0% 0.0 0.2 0.4 0.6 0.8 1.0 1.2 1.4 1.6 1.8 2.0								
Physical Storage Capacity: Depth of Runoff from Impervious Area (inches)								
Total Phosphorus — Volume								



#### What can the manual do for me

Chapter 2 – Retrofit Approach: This chapter defines what a retrofit is and discusses the approaches to identifying and implementing SCMs in a retrofit situation. It reviews the opportunistic approach (including measures as part of other efforts) and the planning approach (proactive planning and prioritization).

### **Retrofit Approach: Planning Approach**

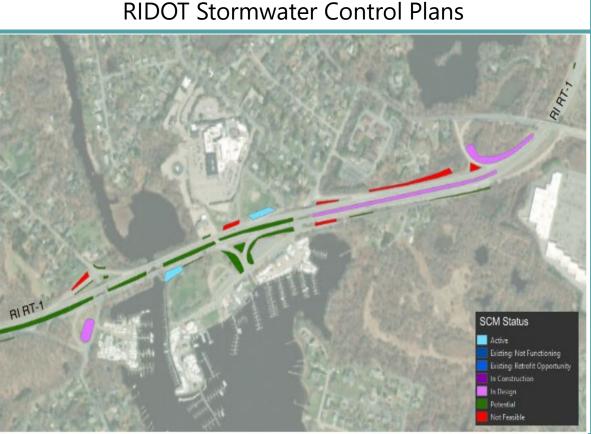
- Proactively planning retrofits and prioritizing sites
- Steps:
  - **1. Understand and Quantify Goals**
  - **2. Identify Potential Sites**
  - 3. Identify SCMs
  - 4. Prioritize Sites and Controls
  - 5. Implement SCMs



Source: https://www.arcgis.com/apps/webappviewer/index.html?id=b516ed62a55847e28d0243ac07206856



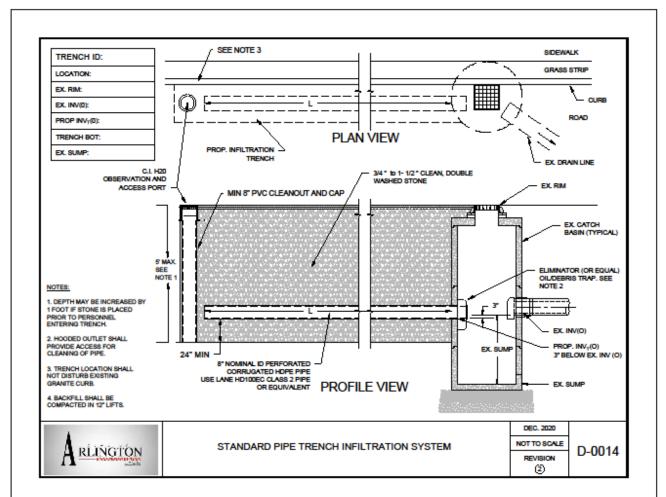




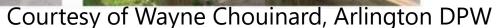
### Planning Approach:













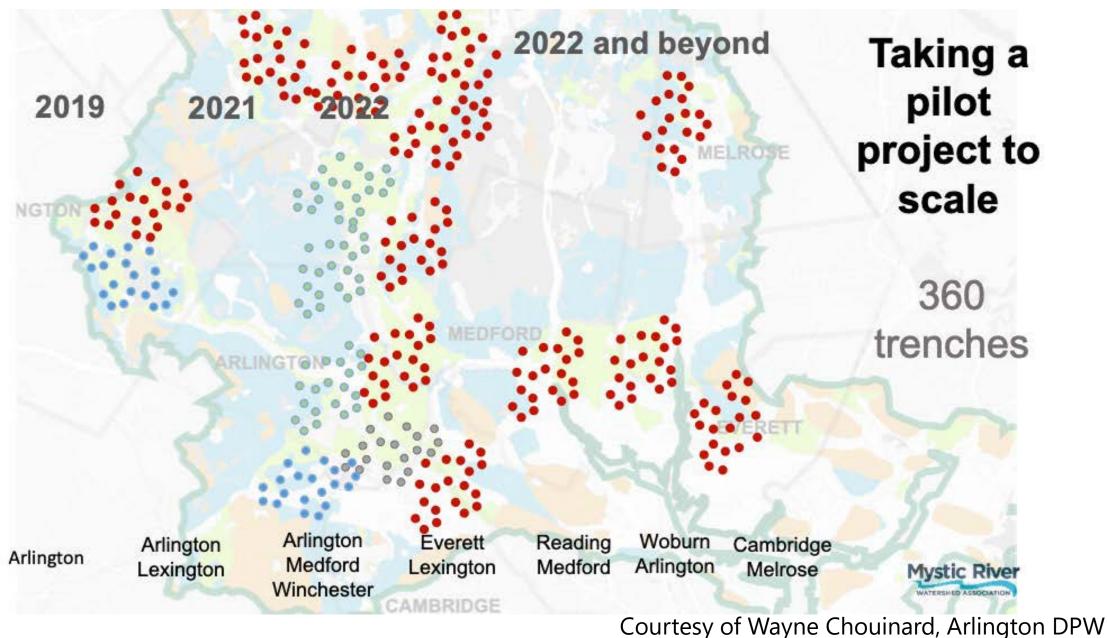
ORMWATER CENTER





#### Courtesy of Wayne Chouinard, Arlington DPW





#### Credits



**Chapter 3 – Credits**: This chapter discusses credits, or the quantification of a SCM's stormwater benefits. It explains the need for credits and presents the SCM Water Quality Treatment Performance Curves, a crediting scheme that is becoming widely accepted in New England for quantifying benefits. The credits show that small scale controls that do not necessarily meet widely implemented sizing standards can still provide significant benefit.

Table 1. Modeled costs and infiltration trench performance*					
Unit	Cost (\$)				
System	2,200				
IC treated per acre (per ha)	18,857 (44,000)				
TP per lb (per kg)	24,750 (55,000)				
TN per lb (per kg)	3,930 (8,609)				
TSS per lb (per kg)	86 (190)				
Volume eliminated per cf (per m³)	0.11 (4)				

#### **Other Benefits**





Checked: JJH

STORMWATER CENTER

http://www.unh.edu/unhsc

Approved:

Feet

Original Drawing Size: 11 x 17 in

Durham, NH

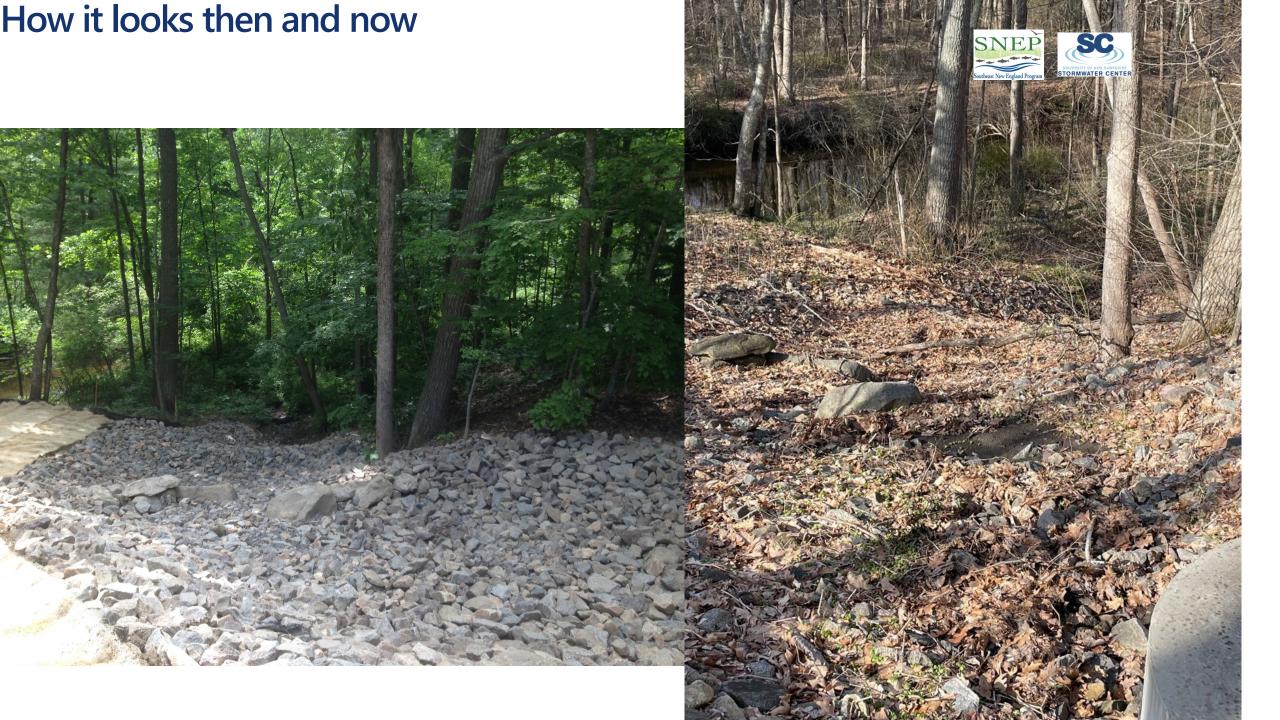
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DA = 10.47 acres IC = 3.14 acresDSV = 1,200 cf PSC = 0.11











**Chapter 4 – Stormwater Control Measures Selection**: This chapter steps through typical unit operations and processes utilized by SCMs. This chapter also provides information on the selection and design process for SCMs and provides sizing guidance. Additional Tools/Resources for SCM selection and sizing are also provided in this chapter.

**Chapter 5- Stormwater Control Measure Guidance** – This chapter steps through the functional components and treatment categories used to construct a SCM. It also steps through SCM variations within each treatment category and provides information on design considerations and Operations and Maintenance (O&M) activities for each variation.

Output	Intermediate	Design Storage Volume	DSV	cf	1,200			
	Calculation 5	Impervious to Pervious Ratio /range cf & 014 - 87	IA/PC	acłac	N/A S	SNEP	SC	<b>Wh</b> b
		Physical Storage Capacity: Depth of Runoff from IA <i>(range of 0 - 2, goal &gt; 0.1, optimal 0.4)</i>	PSC	in	0.11	theast New England Program	UNIVERSITY OF HEW HAMPSHIRE	VIIO,
m 		PSC Notes			DSV small, increase for optimal range 0.3≤ PSC≤ 0.6			
	Performanc	Removal Efficiency: Volume	Vol <sub>re</sub>	-	0%			
	e Curve	Removal Efficiency: P	Pre	-	20%			
	Removal	Removal Efficiency: N	N <sub>re</sub>	-	23%			
	Efficiencies	Removal Efficiency: TSS	TSSre	-	49%			
-		Removal Efficiency: Zn	Zn <sub>re</sub>	-	59%			
		Removal Efficiency: Bacteria	FIBre	-	32%			
	Loading Rate	Load: Volume	Voller	Mgal/yr	3.29			
		Load: P		lbłyr	6.2			
		Load: N		lbłyr	44.3			
		Load: TSS		lbłyr	1,378			
		Load: Bacteria	FIBLER	<b>Billion MPN</b> /y	20			
	Reductions	Reduction: Volume	Vol <sub>Red</sub>	Mgal/yr	0.00			
		Reduction: P	P <sub>R-4</sub>	lb/yr	1.2	1		
_		Reduction: N	N <sub>Red</sub>	lbłyr	10.1			
_		Reduction: TSS	TSSR-4	lbłyr	682	4		
-	<u> </u>	Reduction: Bacteria	FIB <sub>Red</sub>	<u>%łur</u>	32%			
-	Costs	Estimated Total Costs		\$	\$22,000			
-		Removal Costs: Volume Removal Costs: P		\$/Mgal-yr ক#⊾	N/A #17.940			
		Removal Costs: N		\$/lb-yr \$/lb-yr	\$17,940 \$2,170	4		
		Removal Costs: NS		sriu-yi \$∦b-yr	\$30	1		
		Removal Costs: Bacteria		\$7%-yr	\$700	1		
	00M	Estimated O&M Hours		hr/yr	68	j		

### **Other Important Components – Appendix A**

#### **Sizing Requirements**



*Proposed Retrofit Guidance*: New site developments should fully size SCMs. Size retrofit SCMs within existing developed landscapes using the Performance Curves to optimize cost-effective pollutant reduction and encourage the installation of SCMs distributed across the landscape where runoff is generated.

#### **Bedrock and Groundwater Separation for Infiltration SCMs**

*Proposed Retrofit Guidance*: Infiltration systems that include a filtering layer, must have one foot or more of separation from the bottom of the filter course to the SHWT at all times. Can include the filter layer in the groundwater separation calculation.

#### **Soils for Infiltration SCMs**

*Proposed Retrofit Guidance*: Consider infiltration SCMs for all soil groups where infiltration is appropriate.

#### **Pretreatment**

*Proposed Retrofit Guidance*: Provide pretreatment whenever possible. The goal is a viable and accessible maintenance access point.

### Southeast New England Program (SNEP) Network Context



#### 2021 - 2025 PRIORITY ACTIONS

Increase Local Capacity to Complete Projects and Adopt New Policies

#### SNEP Goals

Resilient Ecosystem of Safe and Healthy Waters; Thriving Watersheds and Natural Lands; Sustainable Communities Increase Available Solutions

Demonstrate Ways to Address Common Challenges

Increase Community Leaders' Understanding of the Benefits of Restoration Projects

Source: <u>Southeast New England Program Fact Sheet, February</u> 2021 (epa.gov)

Ensure Diverse Representation

## Southeast New England Program

### Acknowledgments

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